GEOTECHNICS PTY LTD
CONSULTING GEOTECHNICAL ENGINEERS

GEOTECHNICAL INVESTIGATION

FOR

NSW LAND AND HOUSING CORPORATION

310-314 Swan & 984-988 Corella Streets, North Albury, New South Wales (BH2CY)

Report No: 23/3457

Project No: 32427/8053D-G

October 2023

14/1 Cowpasture Place, Wetherill Park, NSW 2164, Australia
(PO Box 6989, Wetherill Park, NSW 2164, Australia)
T +61 2 9756 2166 | **F** +61 2 9756 1137
www.stsgeo.com.au | enquiries@stsgeo.com.au
ABN 45 636 179 729 | **ACN** 636 179 729

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1. INTRODUCTION

This report presents the results of a geotechnical investigation carried out by STS Geotechnics Pty Limited (STS) for the construction of proposed development at 310-314 Swan & 984-988 Corella Streets, North Albury, New South Wales.

Following documents were provided to assist in the preparation of this report:

- Architectural drawings prepared by Planning, Industry & Environment of NSW Government; Plans, Issued for Information, dated April 2023, Project Title '984-988 Corella St & 310-314 Swan St, North Albury', Job reference - BGtbc.

Based on the architectural plans provided, the proposed development will be limited to three storey residential buildings without basement excavation, however, minor cut and fill will be required to achieve the required finished floor levels, footings, and service trenches. There is also a car parking area.

The purpose of this investigation is to determine the following-

- Subsurface conditions,
- Site Classification according to AS2870,
- Foundation design parameters including foundation options,
- Exposure classification in according with AS2870 and AS2159, and
- Pavement thickness.

The investigation was undertaken in accordance with STS proposal P23-430 dated September 19, 2023.

Our scope of work did not include a contamination assessment.

2. NATURE OF THE INVESTIGATION

2.1. Fieldwork

The fieldwork consisted of drilling six (6) boreholes numbered BH1 to BH6 inclusive, at the locations shown on attached Drawing No. 23/3457**.** Boreholes were drilled using a utility mounted Christie drilling rig, owned, and operated by STS. Soils were drilled using rotary solid flight augers. Soil strengths were assessed by carrying out a Dynamic Cone Penetrometer (DCP) tests adjacent to each borehole location.

Drilling operations were undertaken by one of STS's technical officers who also logged the subsurface conditions encountered.

Representative soil samples were collected from the boreholes for subsequent laboratory testing.

The subsurface conditions observed are recorded on the borehole logs given in Appendix A. An explanation of the terms used on the logs is also given in Appendix A. Notes relating to geotechnical reports are also attached.

2.2. Laboratory Testing

To assess the soils for their aggressiveness, selected representative soil samples were tested to determine the following:

- pH,
- Sulphate content (SO_4),
- Chloride content (Cl) and
- Electrical Conductivity (EC).

To assist with determining the site classification, four (4) Shrink Swell tests were carried out on representative samples retrieved from the site.

To assist with determining the pavement thickness, a representative soil sample was taken from the borehole for CBR testing. The representative soil sample was tested to determine the CBR of the natural subgrade onsite. The CBR was compacted to a target ratio of 100% of the Standard Maximum Dry Density (SMDD) and soaked for four (4) days.

Detailed test report is given in Appendix B.

3. GEOLOGY AND SITE CONDITIONS

The Wangaratta geological series sheet at a scale of 1:250,000 shows the site is underlain by Quaternary Age Alluvial Formation. This formation typically comprises gravel, sand, silt, and clay.

At the time of the fieldwork, the site was occupied by single storey houses. Site vegetation comprised grassed lawns, shrubs, and trees. The surface profile falls approximately 1 metre to the south.

The proposed site is bound by Swan Street to the south and Corella Street to the west. Other residential dwellings are present in the adjoining properties.

4. SUBSURFACE CONDITIONS

When assessing the subsurface conditions across a site from a limited number of boreholes, there is the possibility that variations may occur between test locations. The data derived from the site investigation programme are extrapolated across the site to form a geological model and an engineering opinion is rendered about overall subsurface conditions and their likely behaviour regarding the proposed development. The actual condition at the site may differ from those inferred, since no subsurface exploration programme, no matter how comprehensive, can reveal all subsurface details and anomalies, particularly on a site such as this where there has been previous development.

The subsurface conditions consist of fill overlying sandy clays, silty clays, and silty sandy clays. A summary of the subsurface conditions observed are given in Table 4.1

Table 4.1 Subsurface Conditions Summary

Geotechnical Unit	Unit Description	Depth to Top of Unit (m Below Ground Level)					
		BH1	BH2	BH3	BH4	BH5	BH6
Unit 1	**Fill:** Silty Sandy CLAY	0	0	0	0	0	0
Unit 2	Silty Sandy and Sandy CLAY, firm to stiff and very stiff	0.4	-	0.4[1]	0.4[1]	-	-
Unit 3	Silty CLAY, firm to stiff and very stiff	1.7[1]	0.4[1]	-	-	0.3[1]	0.4[1]

- Absent

[1] Present to depth of drilling, 6.0m

Groundwater was not observed during drilling works.

The subsurface conditions observed are recorded on the borehole logs given in Appendix A. An explanation of the terms used on the logs is also given in Appendix A. Notes relating to geotechnical reports are also attached.

5. GEOTECHNICAL DISCUSSION

5.1. Site Classification to AS2870

The classification has been prepared in accordance with the guidelines set out in the "Residential Slabs and Footings" Code, AS2870 - 2011.

To assist with determining the site classification, four (4) shrink/swell tests were carried out on the representative sample retrieved from the site. The detailed testing report is attached and summarised in Table 5.1.

Table 5.1 – Shrink Swell Test Summary

Location	Depth (m)	Material Description	Shrink/Swell Index (% per ΔpF)
BH1	0.6 – 0.9	Sandy Clay, orange brown/yellow	2.7
BH2	0.6 – 0.9	Silty Clay, red/grey brown	2.7
BH3	0.7 – 1.0	Silty Sandy Clay, yellow brown	1.2
BH	0.7 – 1.0	Silty Sandy Clay, yellow brown	3.1

Because there are residential dwellings and trees present, abnormal moisture conditions (AMC) prevail at the site (Refer to Section 1.3.3 of AS2870).

Because of the AMC present, the site is classified a *Problem Site (P).* Provided the recommendations given below are adopted the site may be reclassified *Highly Reactive (H1).*

Foundation design and construction consistent with this classification shall be adopted as specified in the above referenced standard and in accordance with the following design parameters provided below.

5.2. Foundation Design Parameters

We do not recommend founding any structural loads in topsoil and uncontrolled fill.

Pad and/or strip footings founded in natural soils, may be proportioned using an allowable bearing pressure of and 100 kPa. This value may be increased to 200 kPa when founding in the very stiff natural material. The minimum depth of founding must comply with the requirements of AS2870-2011.

Piers founded in very stiff clay materials may be proportioned using an allowable end bearing pressure of 300 kPa, provided their depth to diameter ratio exceeds a value of 4. An allowable adhesion value of 20 kPa may be adopted for the portion of the shaft below a depth of 0.5 metres and within the natural soils.

The site is considered suitable for slab on ground construction provided due regard is given to the ground surface slope.

To ensure the bearing values given can be achieved, care should be taken to ensure the base of the excavations is free of all loose material prior to concreting. To this end, it is recommended that all excavations be concreted as soon as possible, preferably immediately after excavating, cleaning, inspecting and approval. Pier excavations should not be left open overnight. The possibility of groundwater inflow needs to be considered when drilling the piers and pouring concrete.

During foundation construction, should the subsurface conditions vary to those inferred in this report, a suitably experienced geotechnical engineer should review the design and recommendations given above to determine if any alterations are required.

5.3. Pavement Design

The results of the CBR testing for this site is summarised in Table 5.2.

Table 5.2 – CBR Testing Result

Location	Depth (m)	Material Description	CBR (%)
BH 5	0.5 – 0.8	Silty Clay, grey brown, with gravel	5.0

Based on the laboratory test results, a CBR value of 5.0% has been adopted for the design of the pavements in this site.

5.3.1 Flexible Pavement

For the on-grade driveways and carpark areas of the site, the flexible pavement thickness has been determined using the procedures given in Australian Roads Research Board (ARRB) "Sealed Local Roads Manual." We have assumed a 95% confidence level that the pavement will perform satisfactorily during its design life. In the absence of other traffic data, a design traffic loading of $6x10^4$ ESAs is considered appropriate for the site assuming that the driveways and car park are used for light vehicles.

For a design subgrade CBR value of 5.0%, the suggested pavement thickness is a recommended minimum of 370 mm, made up as follows:

Table 5.3 – Flexible Pavement Thickness Design

Material Type	Minimum Thickness (mm)
Surface Course	40
Base Course	100
Subbase Course	230
TOTAL	**370**

5.3.2 Construction

The designs given above assumed adequate provisions have been made for both surface and subsurface water.

The clay soils, which will make up the pavement subgrade, may be reactive. They may therefore be susceptible to shrinkage and swelling due to moisture content changes. If these subgrade soils are permitted to dry following compaction, it is probable that shrinkage will occur, resulting in cracking. After placement of the pavement materials, the subgrade soils may moisten, resulting in swelling and partial loss of strength. It is therefore recommended that the subgrade be covered as soon as possible after completion of compaction to minimise the potential for evaporation and shrinkage to occur.

The subgrade materials should be compacted to a minimum density ratio of 100% of the Standard Maximum Dry Density (SMDD). Compaction should be verified by proof rolling and in-situ density tests. Base and subbase course materials should be compacted and tested to a minimum density ratio of 98% of the Modified Maximum Dry Density (MMDD). The level of compaction should be verified by in-situ density testing.

The long-term successful performance of the pavements is dependent on the satisfactory completion of the earthworks. To achieve this, the quality assurance programme should not be limited to routine compaction density testing only. Other important factors associated with the earthworks includes subgrade preparation, selection of fill materials, control of moisture content and drainage, etc.

All pavement materials used should comply with the Albury City Council requirements.

5.4. Site Preparation and Re-grading

Subgrade Preparation

Earthworks recommendations provided in this report should be complemented by reference to AS3798.

1. The existing fill present on site appears to be suitable to be re-used as engineered fill provided the fill conforms to the requirements of AS3798. Excavations in the silty clay fill may need to be carried out with the excavation sides battered at angles of no steeper than 1(H) to 1(V). The new fill must be 'keyed-in' the sides of these batters. The existing fill material should be removed and if suitable stockpiled for reuse.

2. The base of the excavation should be proof rolled with a smooth drum roller (say 12 tonne) used in static or non-vibratory mode of operation. Caution is required when proof rolling near existing infrastructures and utilities (where present). The purpose of the proof rolling is to detect any soft or heaving areas and to allow for some further improvement in strength or compaction.

3. The final pass should be undertaken in the presence of an experienced geotechnician or geotechnical engineer, to detect any unsuitable or soft subgrade areas, and to allow for some further improvement in strength/compaction.

4. If dry conditions prevail at the time of construction, then any exposed residual clay subgrade may become desiccated or have shrinkage cracks prior to placing pavement materials. If this occurs, the subgrade must be scarified, watered, and recompacted until the cracks are no longer present.

5. Unstable subgrade detected during proof rolling should be locally excavated down to a sound base and replaced with engineered fill or further advice should be sought. Any fill placed to raise site levels should also be engineered fill, as per the specifications below.

Engineered Fill Specifications

For well compacted controlled engineered fill, experience has shown that the long-term settlement due to the self-weight of fill can be up to approximately 1% of the thickness of the fill layer.

Any fill placed for structural purposes should be engineered material. Fill should be compacted in layers not greater than 200 mm loose thickness, to a minimum density ratio of 98% of SMDD with a moisture content within 2% of the optimum moisture content.

The existing clayey soils excavated from cut areas may be reused as engineered fill, provided any unsuitable ('over wet' and 'oversized') and deleterious materials that may be present are removed.

Density tests should be regularly carried out on the fill to confirm the above specifications are achieved in accordance with AS3798-2007 Table 8.1. We recommend that Level 1 control of fill compaction, as defined in AS3798-2007, be adopted.

We recommend that the engineered fill layers extend a horizontal distance of at least 1m beyond the design geometry. The roller must extend over the edge of each placed layer to seal the batter surface. On completion of filling, the excess under-compacted edge fill should be trimmed back to the design geometry.

The 'tying in' of engineered fill to temporary cut batter slopes can be achieved by locally benching the cut slopes in no greater than 0.4m high steps. This can be carried out progressively as the height of engineered fill increases.

For backfilling confined excavations such as service trenches, a similar compaction to engineered fill should be adhered to, but if light compaction equipment is used then the layer thickness should be limited to 100mm loose thickness.

During construction of the fill, platform runoff should be enhanced by providing suitable falls to reduce ponding of water on the surface of the fill. Ponding of water may lead to softening of the fill and subsequent delays in the earthworks program. A poorly drained clayey subgrade will likely become un-trafficable when wet. We recommend that if soil softening occurs, the exposed surface be over-excavated to below the affected soil, and then replaced with engineered fill as specified above.

5.5. Soil Aggressiveness and Salinity

The aggressiveness or erosion potential of an environment in building materials, particularly concrete and steel is dependent on the levels of soil pH and the types of salts present, generally sulfates and chlorides. To determine the degree of aggressiveness, the test values obtained are compared to Tables 6.4.2 (C) and 6.5.2 (C) in AS2159 – 2009 Piling – Design and Installation. The test results are summarised in Table 5.3.

Table 5.3 – Soil Aggressiveness Summary

Sample No.	Location	Depth (m)	pH	Sulfate (mg/kg)	Chloride (mg/kg)	Electrical Conductivity (dS/m)	
						$EC_{1:5}$	EC_e
S1	BH1	0.3	6.3	<10	<10	0.022	0.2
S2	BH2	0.3	6.1	<10	<10	0.024	0.2
S3	BH3	0.3	6.5	<10	<10	0.011	0.1
S4	BH4	0.3	6.6	<10	<10	0.023	0.2

The soils samples were cohesive and above groundwater. Therefore, soil conditions B are considered appropriate (AS2159).

A review of the durability aspects indicates that:

- pH : minimum value of 6.1
- SO_4 : maximum value of <10 mg/kg (ppm) < 5000 ppm
- Cl : maximum value of <10 mg/kg (ppm) < 5000 ppm
- EC_e : maximum value of 0.2 dS/m

In accordance with AS2159-2009 the exposure classification for the onsite soils is non-aggressive to both concrete and steel. In accordance with AS2870-2011 the soils are classified as A1.

Reference to DLWC (2002) "Site Investigations for Urban Salinity" indicates that an EC_e value of 0.2 dS/m is consistent with the presence of non-saline soils.

6. FINAL COMMENTS

During construction, should the subsurface conditions vary from those inferred above, we would be contacted to determine if any changes should be made to our recommendations. The exposed bearing surfaces for footings should be inspected by a geotechnical engineer to ensure the allowable pressure given has been achieved.

The above classification has been made assuming that all footings will bear in either natural ground or in controlled filling. Prior to the placement of any filling the existing surface should be stripped of all vegetation and topsoil.

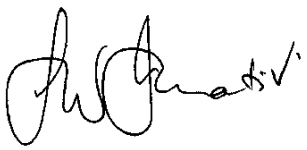
If excavations for rainwater or detention tanks are to be made within 6 metres of the building foundations, advice should be sought regarding their effect on the foundations.

Placing absorption trenches on the high side of the property may create abnormal moisture conditions for the foundations (Refer to Section 1.3.3 of AS2870). This could have a negative effect on the foundation performance and more than likely alter the site classification provided above.

This report has been prepared assuming that no trees other than those noted will be present on the site. If future tree planting is planned, eg. there is a landscaping plan, their effect on the foundation performance must be considered.

This report has been prepared assuming the site development will be limited to three storey residential buildings. The information and interpretation may not be relevant if the design proposal changes (e.g. to a five-storey building involving major cuts during the site preparation). If changes occur, we would be pleased to review the report and advise on the adequacy of the investigation.

Manoj Shrestha
MIE Aust
Geotechnical Engineer
STS Geotechnics Pty Limited



Laurie Ihnativ
Principal Geotechnical Engineer
STS Geotechnics Pty Limited

N

SITE SURROUNDED BY TREES

STS Geotechnics Pty. Ltd.	Scale: Unknown	Date: October 2023
Client: NSW LAND & HOSUING COROPERATION		
GEOTECHNICAL INVESTIGATION—310-314 SWAN & 984– 988 CORRELLA STREETS, NORTH ALBURY BOREHOLE AND PENETROMETER LOCATIONS	Project No. 32427/8053D-G	
	Drawing No: 23/3457	

Important Information

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INTRODUCTION

These notes have been provided to outline the methodology and limitations inherent in geotechnical reporting. The issues discussed are not relevant to all reports and further advice should be sought if there are any queries regarding any advice or report. When copies of reports are made, they should be reproduced in full.

GEOTECHNICAL REPORTS

Geotechnical reports are prepared by qualified personnel on the information supplied or obtained and are based on current engineering standards of interpretation and analysis.

Information may be gained from limited subsurface testing, surface observations, previous work and is supplemented by knowledge of the local geology and experience of the range of properties that may be exhibited by the materials present. For this reason, geotechnical reports should be regarded as interpretative rather than factual documents, limited to some extent by the scope of information on which they rely.

Where the report has been prepared for a specific purpose (eg. design of a three-storey building), the information and interpretation may not be appropriate if the design is changed (eg. a twenty storey building). In such cases, the report and the sufficiency of the existing work should be reviewed by STS Geotechnics Pty Limited in the light of the new proposal.

Every care is taken with the report content, however, it is not always possible to anticipate or assume responsibility for the following conditions:

- Unexpected variations in ground conditions. The potential for this depends on the amount of investigative work undertaken.
- Changes in policy or interpretation by statutory authorities.
- The actions of contractors responding to commercial pressures.

If these occur, STS Geotechnics Pty Limited would be pleased to resolve the matter through further investigation, analysis or advice.

UNFORSEEN CONDITIONS

Should conditions encountered on site differ markedly from those anticipated from the information contained in the report, STS Geotechnics Pty Limited should be notified immediately. Early identification of site anomalies generally results in any problems being more readily resolved and allows re-interpretation and assessment of the implications for future work.

SUBSURFACE CONDITIONS

Logs of a borehole, recovered core, test pit, excavated face or cone penetration test are an engineering and/or geological interpretation of the subsurface conditions. The reliability of the logged information depends on the drilling/testing method, sampling and/or observation spacings and the ground conditions. It is not always possible or economic to obtain continuous high quality data. It should also be recognised that the volume or material observed or tested is only a fraction of the total subsurface profile.

Interpretation of subsurface information and application to design and construction must take into consideration the spacing of the test locations, the frequency of observations and testing, and the possibility that geological boundaries may vary between observation points.

Groundwater observations and measurements outside of specially designed and constructed piezometers should be treated with care for the following reasons:

- In low permeability soils groundwater may not seep into an excavation or bore in the short time it is left open.
- A localised perched water table may not represent the true water table.
- Groundwater levels vary according to rainfall events or season.
- Some drilling and testing procedures mask or prevent groundwater inflow.

The installation of piezometers and long term monitoring of groundwater levels may be required to adequately identify groundwater conditions.

SUPPLY OF GETEOECHNICAL INFORMATION OR TENDERING PURPOSES

It is recommended tenderers are provided with as much geological and geotechnical information that is available and that where there are uncertainties regarding the ground conditions, prospective tenders should be provided with comments discussing the range of likely conditions in addition to the investigation data.

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APPENDIX A – BOREHOLE LOGS AND EXPLANATION SHEETS

STS Geotechnics Pty Ltd

GEOTECHNICAL LOG - NON CORE BOREHOLE

Client: NSW Land & Housing Corporation	Project: 32427/8053D-G	**BOREHOLE NO.: BH 1**
Project: 310-314 Swan & 984-988 Corella Streets, North Albury	Date : October 9, 2023	
Location: Refer to Drawing No. 23/3457	Logged: PS Checked By: MT	Sheet 1 of 1

WATER TABLE	SAMPLES	DEPTH (m)	DESCRIPTION OF DRILLED PRODUCT Soil Name, grain size /plasticity, colour; secondary constituents (Inc. Description) , minor constituents including other remarks	SYMBOL	CONSISTENCY (cohesive soils) or RELATIVE DENSITY (sands and gravels)	MOISTURE
	S1 @ 0.3 m		FILL: SILTY SANDY CLAY: low to medium plasticity, grey, trace of gravel and rootlets	CL/CI		<PL
	U50	1.0	SANDY CLAY: low to medium plasticity, grey orange yellow brown, with gravel	CL/CI	STIFF TO VERY STIFF	<PL
		2.0	SILTY CLAY: grey, medium to high plasticity	CI/CH		<PL
		3.0				
		4.0				
		5.0				
			BOREHOLE DISCONTINUED AT 6.0 M			

D - disturbed sample
U - undisturbed tube sample
B - bulk sample
WT - level of water table or free water
N - Standard Penetration Test (SPT)
S - jar sample

Contractor: STS
Equipment: Christie
Hole Diameter (mm): 100
Angle from Vertical (°): 0
Drill Bit: Spiral

NOTES: See explanation sheets for meaning of all descriptive terms and symbols

STS Geotechnics Pty Ltd

GEOTECHNICAL LOG - NON CORE BOREHOLE

Client: NSW Land & Housing Corporation	Project: 32427/8053D-G	**BOREHOLE NO.:**	**BH 2**
Project: 310-314 Swan & 984-988 Corella Streets, North Albury	Date : October 9, 2023	Sheet 1 of 1	
Location: Refer to Drawing No. 23/3457	Logged: PS Checked By: MT		

WATER TABLE	SAMPLES	DEPTH (m)	DESCRIPTION OF DRILLED PRODUCT Soil Name, grain size /plasticity, colour; secondary constituents (Inc. Description) , minor constituents including other remarks	SYMBOL	CONSISTENCY (cohesive soils) or RELATIVE DENSITY (sands and gravels)	MOISTURE
	S2 @ 0.3 m		FILL: SILTY SANDY CLAY: low plasticity, grey, with rootlets	CL/CI		<PL
	U50	1.0	SILTY CLAY: low to medium plasticity, red grey brown	CL/CI	STIFF TO VERY STIFF	<PL
		2.0				
		3.0				
		4.0				
		5.0				
			BOREHOLE DISCONTINUED AT 6.0 M			

D - disturbed sample U - undisturbed tube sample B - bulk sample
WT - level of water table or free water N - Standard Penetration Test (SPT)
S - jar sample

Contractor: STS
Equipment: Christie
Hole Diameter (mm): 100
Angle from Vertical (^{o}): 0
Drill Bit: Spiral

NOTES: See explanation sheets for meaning of all descriptive terms and symbols

STS Geotechnics Pty Ltd

GEOTECHNICAL LOG - NON CORE BOREHOLE

Client: NSW Land & Housing Corporation	Project: 32427/8053D-G	**BOREHOLE NO.:**	**BH 3**
Project: 310-314 Swan & 984-988 Corella Streets, North Albury	Date : October 9, 2023	Sheet 1 of 1	
Location: Refer to Drawing No. 23/3457	Logged: PS Checked By: MT		

WATER TABLE	SAMPLES	DEPTH (m)	DESCRIPTION OF DRILLED PRODUCT Soil Name, grain size /plasticity, colour; secondary constituents (Inc. Description) , minor constituents including other remarks	SYMBOL	CONSISTENCY (cohesive soils) or RELATIVE DENSITY (sands and gravels)	MOISTURE
	S3 @ 0.3 m		FILL: SILTY SANDY CLAY: low plasticity, grey, fine grained sand, with rootlets	CL		≤PL
	U50	1.0	SILTY SANDY CLAY: low to medium plasticity, grey yellow brown	CL/CI	FIRM TO STIFF BECOMING STIFF VERY STIFF	≤PL <PL
		2.0				
		3.0				
		4.0				
		5.0				
			BOREHOLE DISCONTINUED AT 6.0 M			

D - disturbed sample U - undisturbed tube sample B - bulk sample
WT - level of water table or free water N - Standard Penetration Test (SPT)
S - jar sample

NOTES: See explanation sheets for meaning of all descriptive terms and symbols

Contractor: STS
Equipment: Christie
Hole Diameter (mm): 100
Angle from Vertical (^{o}): 0
Drill Bit: Spiral

STS Geotechnics Pty Ltd

GEOTECHNICAL LOG - NON CORE BOREHOLE

Client: NSW Land & Housing Corporation	Project: 32427/8053D-G	**BOREHOLE NO.:**	**BH 4**
Project: 310-314 Swan & 984-988 Corella Streets, North Albury	Date : October 9, 2023	Sheet 1 of 1	
Location: Refer to Drawing No. 23/3457	Logged: PS Checked By: MT		

WATER TABLE	SAMPLES	DEPTH (m)	DESCRIPTION OF DRILLED PRODUCT Soil Name, grain size /plasticity, colour; secondary constituents (Inc. Description) , minor constituents including other remarks	SYMBOL	CONSISTENCY (cohesive soils) or RELATIVE DENSITY (sands and gravels)	MOISTURE
	S4 @ 0.3 M		FILL: SILTY SANDY CLAY: lowto Medium plasticity, grey brown, with rootlets	CL/CI		≤PL
	U50	1.0	SILTY SANDY CLAY: low to medium plasticity, grey yellow brown	CL/CI	FIRM TO STIFF BECOMING STIFF	≥PL
		2.0			VERY STIFF	
		3.0				
		4.0				
		5.0				
			BOREHOLE DISCONTINUED AT 6.0 M			

D - disturbed sample U - undisturbed tube sample B - bulk sample
WT - level of water table or free water N - Standard Penetration Test (SPT)
S - jar sample

Contractor: STS
Equipment: Christie
Hole Diameter (mm): 100
Angle from Vertical (^{o}): 0
Drill Bit: Spiral

NOTES: See explanation sheets for meaning of all descriptive terms and symbols

STS Geotechnics Pty Ltd

GEOTECHNICAL LOG - NON CORE BOREHOLE

Client: NSW Land & Housing Corporation	Project: 32427/8053D-G	**BOREHOLE NO.: BH 5**
Project: 310-314 Swan & 984-988 Corella Streets, North Albury	Date : October 9, 2023	
Location: Refer to Drawing No. 23/3457	Logged: PS Checked By: MT	Sheet 1 of 1

WATER TABLE	SAMPLES	DEPTH (m)	DESCRIPTION OF DRILLED PRODUCT Soil Name, grain size /plasticity, colour; secondary constituents (Inc. Description) , minor constituents including other remarks	SYMBOL	CONSISTENCY (cohesive soils) or RELATIVE DENSITY (sands and gravels)	MOISTURE
			FILL: SILTY SANDY CLAY: low plasticity, grey, with rootlets	CL		≤PL
			SILTY CLAY; medium plasticity, grey, brown	CI	FIRM TO STIFF	≤PL
	B				BECOMING STIFF	
		1.0			VERY STIFF	<PI
		2.0				
		3.0				
		4.0				
		5.0				
			BOREHOLE DISCONTINUED AT 6.0 M			

D - disturbed sample U - undisturbed tube sample B - bulk sample
WT - level of water table or free water N - Standard Penetration Test (SPT)
S - jar sample

NOTES: See explanation sheets for meaning of all descriptive terms and symbols

Contractor: STS
Equipment: Christie
Hole Diameter (mm): 100
Angle from Vertical (^{o}): 0
Drill Bit: Spiral

STS Geotechnics Pty Ltd

GEOTECHNICAL LOG - NON CORE BOREHOLE

Client: NSW Land & Housing Corporattion	Project: 32427/8053D-G	**BOREHOLE NO.: BH 6**
Project: 310-314 Swan & 984-988 Corella Streets, North Albury	Date : October 9, 2023	
Location: Refer to Drawing No. 23/3457	Logged: PS Checked By: MT	Sheet 1 of 1

WATER TABLE	SAMPLES	DEPTH (m)	DESCRIPTION OF DRILLED PRODUCT Soil Name, grain size /plasticity, colour; secondary constituents (Inc. Description) , minor constituents including other remarks	SYMBOL	CONSISTENCY (cohesive soils) or RELATIVE DENSITY (sands and gravels)	MOISTURE
			FILL: SILTY SANDY CLAY: low plasticity, grey/brown, with rootlets	CL		=PL
			SILTY CLAY: medium plasticity, brown	CI	FIRM TO STIFF	≤PL
		1.0			VERY STIFF	<PL
		2.0				
		3.0				
		4.0				
		5.0				
			BOREHOLE DISCONTINUED AT 6.0 M			

D - disturbed sample U - undisturbed tube sample B - bulk sample
WT - level of water table or free water N - Standard Penetration Test (SPT)
S - jar sample

NOTES: See explanation sheets for meaning of all descriptive terms and symbols

Contractor: STS
Equipment: Christie
Hole Diameter (mm): 100
Angle from Vertical (^{o}): 0
Drill Bit: Spiral

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STS Geotechnics Pty Ltd
14/1 Cowpasture Place, Wetherill Park NSW 2164
Phone: (02)9756 2166 | Email: enquiries@stsgeo.com.au

Dynamic Cone Penetrometer Test Report

Project: 310-314 SWAN & 984-988 CORELLA STREETS, NORTH ALBURY — Project No.: 32427/8053D-G

Client: NSW LAND & HOUSING CORPORATION — Report No.: 23/3456

Address: 4 Parmatta Square, 12 Darcy Street, Paramatta — Report Date: 16/10/2023

Test Method: AS 1289.6.3.2 — Page: 1 of 1

Site No.	P1	P2	P3	P4	P5	P6
Location	Refer to Drawing No. 23/3457	Refer to Drawing No. 23/3457	Refer to Drawing No. 23/3457	Refer to Drawing No. 23/3457	Refer to Drawing No. 23/3457	Refer to Drawing No. 23/3457
Date Tested	9/10/2023	9/10/2023	9/10/2023	9/10/2023	9/10/2023	9/10/2023
Starting Level	Surface Level	Surface Level	Surface Level	Surface Level	Surface Level	Surface Level
Depth (m)	Penetration Resistance (blows / 150mm)					
0.00 - 0.15	2	2	1	1	1	1
0.15 - 0.30	4	4	1	4	4	1
0.30 - 0.45	5	7	1	3	4	2
0.45 - 0.60	13	16	2	2	4	4
0.60 - 0.75	15	16	3	3	4	6
0.75 - 0.90	22	9	4	5	5	7
0.90 - 1.05	23+	10	6	6	5	10
1.05 - 1.20	Discontinued	10	13	7	7	6
1.20 - 1.35		9	17	7	9	7
1.35 - 1.50		8	19	8	11	8
1.50 - 1.65		9	23+	14	11	12
1.65 - 1.80		10	Discontinued	19	14	19
1.80 - 1.95		8		23+	23+	23+
1.95 - 2.10		11		Discontinued	Discontinued	Discontinued
2.10 - 2.25		11				
2.25 - 2.40		13				
2.40 - 2.55		18				
2.55 - 2.70		Discontinued				
2.70 - 2.85						
2.85 - 3.00						
3.00 - 3.15						
3.15 - 3.30						
3.30 - 3.45						
3.45 - 3.60						
3.60 - 3.75						

Remarks: * Pre drilled prior to testing

Approved Signatory.....

Technician: PS — Orlando Mendoza - Laboratory Manager

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EXPLANATION OF NOTES, ABBREVIATIONS & TERMS USED ON BOREHOLE AND TEST PIT LOGS

DRILLING/EXCAVATION METHOD

HA	Hand Auger	**ADH**	Hollow Auger	**NQ**	Diamond Core - 47 mm
DT	Diatube Coring	**RT**	Rotary Tricone bit	**NMLC**	Diamond Core - 52 mm
NDD	Non-destructive digging	**RAB**	Rotary Air Blast	**HQ**	Diamond Core - 63 mm
AD*	Auger Drilling	**RC**	Reverse Circulation	**HMLC**	Diamond Core - 63 mm
***V**	V-Bit	**PT**	Push Tube	**EX**	Tracked Hydraulic Excavator
***T**	TC-Bit, e.g. AD/T	**WB**	Washbore	**HAND**	Excavated by Hand Methods

PENETRATION RESISTANCE

L	**Low Resistance**	Rapid penetration/ excavation possible with little effort from equipment used.
M	**Medium Resistance**	Penetration/ excavation possible at an acceptable rate with moderate effort from equipment used.
H	**High Resistance**	Penetration/ excavation is possible but at a slow rate and requires significant effort from equipment used.
R	**Refusal/Practical Refusal**	No further progress possible without risk of damage or unacceptable wear to equipment used.

These assessments are subjective and are dependent on many factors, including equipment power and weight, condition of excavation or drilling tools and experience of the operator.

WATER

Standing Water Level **Partial water loss**

Water Seepage **Complete Water Loss**

GWNO	GROUNDWATER NOT OBSERVED - Observation of groundwater, whether present or not, was not possible due to drilling water, surface seepage or cave-in of the borehole/ test pit.
GWNE	GROUNDWATER NOT ENCOUNTERED - Borehole/ test pit was dry soon after excavation. However, groundwater could be present in less permeable strata. Inflow may have been observed had the borehole/ test pit been left open for a longer period.

SAMPLING AND TESTING

SPT	Standard Penetration Testing to AS1289.6.3.3 2004
4,7,11 N=18	4,7,11 = Blows per 150mm. N = Blows per 300mm penetration following a 150mm seating drive
30/80mm	Where practical refusal occurs, the blows and penetration for that interval are reported, N is not reported
RW	Penetration occurred under the rod weight only, N<1
HW	Penetration occurred under the hammer and rod weight only, N<1
HB	Hammer double bouncing on anvil, N is not reported
Sampling	
S1	Jar sample – number indicates sample number
D	Disturbed Sample
B	Bulk disturbed Sample
U50	Thin walled tube sample - number indicates nominal sample diameter in millimetres
Testing	
PP	Pocket Penetrometer test expressed as instrument reading in kPa
DCP	Dynamic Cone Penetrometer (AS1289.6.3.1 1997)
PSP	Perth Sand Penetrometer (AS1289.6.3.2 1997)

GEOLOGICAL BOUNDARIES

———— = Observed Boundary (Position known)

– – – – – – – – – – = Observed Boundary (Position approximate)

– –?– –?– –?– – = Boundary (Interpreted or inferred)

ROCK CORE RECOVERY

TCR =Total Core Recovery (%)

$$= \frac{Length\ of\ core\ recovered}{Length\ of\ core\ run} \times 100$$

RQD = Rock Quality Designation (%)

$$= \frac{\sum Axial\ lengths\ of\ core > 100mm}{Length\ of\ core\ run} \times 100$$

METHOD OF SOIL DESCRIPTION USED ON BOREHOLE AND TEST PIT LOGS

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FILL

COUBLES or BOULDERS

GRAVEL (GP or GW)

ORGANIC SOILS (OL, OH or Pt)

SILT (ML or MH)

CLAY (CL, CI or CH)

SAND (SP or SW)

Combinations of these basic symbols may be used to indicate mixed materials such as sandy clay

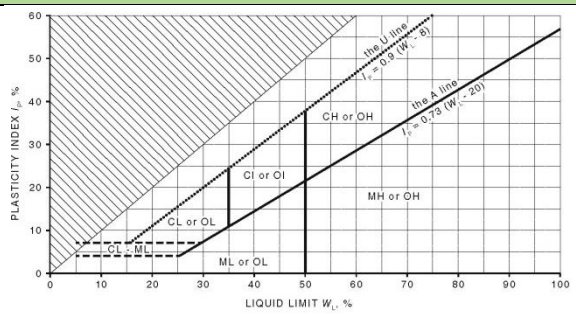
CLASSIFICATION AND INFERRED STRATIGRAPHY

Soil is broadly classified and described in Borehole and Test Pit Logs using the preferred method given in AS 1726:2017, Section 6.1 – Soil description and classification.

PARTICLE SIZE CHARACTERISTICS

Fraction	Components	Sub Division	Size mm
Oversize	BOULDERS		>200
	COBBLES		63 to 200
Coarse grained soil	GRAVEL	Coarse	19 to 63
		Medium	6.7 to 19
		Fine	2.36 to 6.7
	SAND	Coarse	0.6 to 2.36
		Medium	0.21 to 0.6
		Fine	0.075 to 0.21
Fine grained soil	SILT		0.002 to 0.075
	CLAY		<0.002

PLASTICITY PROPERTIES



GROUP SYMBOLS

Major Divisions		Symbol	Description
COARSE GRAINED SOILS More than 65% of soil excluding oversize fraction is greater than 0.075mm	GRAVEL More than 50% of coarse fraction is >2.36mm	GW	Well graded gravel and gravel-sand mixtures, little or no fines, no dry strength.
		GP	Poorly graded gravel and gravel-sand mixtures, little or no fines, no dry strength.
		GM	Silty gravel, gravel-sand-silt mixtures, zero to medium dry strength.
		GC	Clayey gravel, gravel-sand-clay mixtures, medium to high dry strength.
	SAND More than 50% of coarse fraction is <2.36 mm	SW	Well graded sand and gravelly sand, little or no fines, no dry strength.
		SP	Poorly graded sand and gravelly sand, little or no fines, no dry strength.
		SM	Silty sand, sand-silt mixtures, zero to medium dry strength.
		SC	Clayey sand, sandy-clay mixtures, medium to high dry strength.
FINE GRAINED SOILS More than 35% of soil excluding oversized fraction is less than 0.075mm	Liquid Limit less < 50%	ML	Inorganic silts of low plasticity, very fine sands, rock flour, silty or clayey fine sands, zero to medium dry strength.
		CL, CI	Inorganic clays of low to medium plasticity, gravelly clays, sandy clays, silty clays, medium to high dry strength.
		OL	Organic silts and organic silty clays of low plasticity, low to medium dry strength.
	Liquid Limit > than 50%	MH	Inorganic silts of high plasticity, high to very high dry strength.
		CH	Inorganic clays of high plasticity, high to very high dry strength.
		OH	Organic clays of medium to high plasticity, medium to high dry strength.
Highly Organic soil		PT	Peat muck and other highly organic soils.

MOISTURE CONDITION

Symbol	Term	Description
D	Dry	Non- cohesive and free running.
M	Moist	Soils feel cool, darkened in colour. Soil tends to stick together.
W	Wet	Soils feel cool, darkened in colour. Soil tends to stick together, free water forms when handling.

Moisture content of cohesive soils shall be described in relation to plastic limit (PL) or liquid limit (LL) for soils with higher moisture content as follows: Moist, dry of plastic limit (w < PL); Moist, near plastic limit ($w \approx$ PL); Moist, wet of plastic limit (w < PL); Wet, near liquid limit ($w \approx$ LL), Wet, wet of liquid limit (w > LL),

CONSISTENCY

Symbol	Term	Undrained Shear Strength (kPa)	SPT "N" #
VS	Very Soft	≤ 12	≤ 2
S	Soft	>12 to ≤ 25	>2 to ≤ 4
F	Firm	>25 to ≤ 50	>4 to 8
St	Stiff	>50 to ≤ 100	>8 to 15
VSt	Very Stiff	>100 to ≤ 200	>15 to 30
H	Hard	>200	>30
Fr	Friable	-	

DENSITY

Symbol	Term	Density Index %	SPT "N" #
VL	Very Loose	≤ 15	0 to 4
L	Loose	>15 to ≤ 35	4 to 10
MD	Medium Dense	>35 to ≤ 65	10 to 30
D	Dense	>65 to ≤ 85	30 to 50
VD	Very Dense	>85	Above 50

In the absence of test results, consistency and density may be assessed from correlations with the observed behaviour of the material. # SPT correlations are not stated in AS1726:2017, and may be subject to corrections for overburden pressure, moisture content of the soil, and equipment type.

MINOR COMPONENTS

Term	Assessment Guide	Proportion by Mass
Add 'Trace'	Presence just detectable by feel or eye but soil properties little or no different to general properties of primary component	Coarse grained soils: ≤ 5% Fine grained soil: ≤ 15%
Add 'With'	Presence easily detectable by feel or eye but soil properties little or no different to general properties of primary component	Coarse grained soils: 5 - 12% Fine grained soil: 15 - 30%
Prefix soil name	Presence easily detectable by feel or eye in conjunction with the general properties of primary component	Coarse grained soils: >12% Fine grained soil: >30%

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TERMS FOR ROCK MATERIAL STRENGTH AND WEATHERING

CLASSIFICATION AND INFERRED STRATIGRAPHY

Rock is broadly classified and described in Borehole and Test Pit Logs using the preferred method given in AS1726 – 2017, Section 6.2 – Rock identification, description and classification.

ROCK MATERIAL STRENGTH CLASSIFICATION

Symbol	Term	Point Load Index, $Is_{(50)}$ (MPa) #	Field Guide
VL	Very Low	0.03 to 0.1	Material crumbles under firm blows with sharp end of pick; can be peeled with knife; too hard to cut a triaxial sample by hand. Pieces up to 30 mm can be broken by finger pressure.
L	Low	0.1 to 0.3	Easily scored with a knife; indentations 1 mm to 3 mm show in the specimen with firm blows of pick point; has dull sound under hammer. A piece of core 150 mm long by 50 mm diameter may be broken by hand. Sharp edges of core may be friable and break during handling.
M	Medium	0.3 to 1	Readily scored with a knife; a piece of core 150 mm long by 50 mm diameter can be broken by hand with difficulty.
H	High	1 to 3	A piece of core 150 mm long by 50 mm diameter cannot be broken by hand but can be broken with pick with a single firm blow; rock rings under hammer.
VH	Very High	3 to 10	Hand specimen breaks with pick after more than one blow; rock rings under hammer.
EH	Extremely High	>10	Specimen requires many blows with geological pick to break through intact material; rock rings under hammer.

 # Rock Strength Test Results

▼ Point Load Strength Index, $Is_{(50)}$, Axial test (MPa)

● Point Load Strength Index, $Is_{(50)}$, Diametral test (MPa)

Relationship between rock strength test result ($Is_{(50)}$) and unconfined compressive strength (UCS) will vary with rock type and strength, and should be determined on a site-specific basis. However UCS is typically 20 x $Is_{(50)}$.

ROCK MATERIAL WEATHERING CLASSIFICATION

Symbol		Term	Field Guide
RS		Residual Soil	Soil developed on extremely weathered rock; the mass structure and substance fabric are no longer evident; there is a large change in volume but the soil has not been significantly transported.
XW		Extremely Weathered	Rock is weathered to such an extent that it has soil properties - i.e. it either disintegrates or can be remoulded, in water.
DW	HW	Distinctly Weathered	Rock strength usually changed by weathering. The rock may be highly discoloured, usually by iron staining. Porosity may be increased by leaching, or may be decreased due to deposition of weathering products in pores. In some environments it is convenient to subdivide into Highly Weathered and Moderately Weathered, with the degree of alteration typically less for MW.
	MW		
SW		Slightly Weathered	Rock slightly discoloured but shows little or no change of strength relative to fresh rock.
FR		Fresh	Rock shows no sign of decomposition or staining.

ABBREVIATIONS AND DESCRIPTIONS FOR ROCK MATERIAL AND DEFECTS

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CLASSIFICATION AND INFERRED STRATIGRAPHY

Rock is broadly classified and described in Borehole and Test Pit Logs using the preferred method given in AS1726 – 2017, Section 6.2 – Rock identification, description and classification.

DETAILED ROCK DEFECT SPACING

Defect Spacing

Spacing/width (mm)	Descriptor	Symbol
<20	Extremely Close	EC
20-60	Very Close	VC
60-200	Close	C
200-600	Medium	M
600-2000	Wide	W
2000-6000	Very Wide	VW

Bedding Thickness (Stratification)

Term	Spacing (mm)
Thinly laminated	<6
Laminated	6 – 20
Very thinly bedded	20 – 60
Thinly bedded	60 – 200
Medium bedded	200 – 600
Thickly bedded	600 – 2,000
Very thickly bedded	> 2,000

ABBREVIATIONS AND DESCRIPTIONS FOR DEFECT TYPES

Defect Type	Abbr.	Description
Joint	JT	Surface of a fracture or parting, formed without displacement, across which the rock has little or no tensile strength. May be closed or filled by air, water or soil or rock substance, which acts as cement.
Bedding Parting	BP	Surface of fracture or parting, across which the rock has little or no tensile strength, parallel or sub-parallel to layering/ bedding. Bedding refers to the layering or stratification of a rock, indicating orientation during deposition, resulting in planar anisotropy in the rock material.
Contact	CO	The surface between two types or ages of rock.
Sheared Surface	SSU	A near planar, curved or undulating surface which is usually smooth, polished or slickensided.
Sheared Seam/ Zone (Fault)	SS/SZ	Seam or zone with roughly parallel almost planar boundaries of rock substance cut by closely spaced (often <50 mm) parallel and usually smooth or slickensided joints or cleavage planes.
Crushed Seam/ Zone (Fault)	CS/CZ	Seam or zone composed of disoriented usually angular fragments of the host rock substance, with roughly parallel near-planar boundaries. The brecciated fragments may be of clay, silt, sand or gravel sizes or mixtures of these.
Extremely Weathered Seam/ Zone	XWS/XWZ	Seam of soil substance, often with gradational boundaries, formed by weathering of the rock material in places.
Infilled Seam	IS	Seam of soil substance, usually clay or clayey, with very distinct roughly parallel boundaries, formed by soil migrating into joint or open cavity.
Vein	VN	Distinct sheet-like body of minerals crystallised within rock through typically open-space filling or crack-seal growth.

NOTE: Defects size of <100mm SS, CS and XWS. Defects size of >100mm SZ, CZ and XWZ.

ABBREVIATIONS AND DESCRIPTIONS FOR DEFECT SHAPE AND ROUGHNESS

Shape	Abbr.	Description	Roughness	Abbr.	Description
Planar	PR	Consistent orientation	Polished	POL	Shiny smooth surface
Curved	CU	Gradual change in orientation	Slickensided	SL	Grooved or striated surface, usually polished
Undulating	UN	Wavy surface	Smooth	SM	Smooth to touch. Few or no surface irregularities
Stepped	ST	One or more well defined steps	Rough	RO	Many small surface irregularities (amplitude generally <1mm). Feels like fine to coarse sandpaper
Irregular	IR	Many sharp changes in orientation	Very Rough	VR	Many large surface irregularities, amplitude generally >1mm. Feels like very coarse sandpaper

Orientation: **Vertical Boreholes –** The dip (inclination from horizontal) of the defect.
Inclined Boreholes – The inclination is measured as the acute angle to the core axis.

ABBREVIATIONS AND DESCRIPTIONS FOR DEFECT COATING

Coating	Abbr.	Description
Clean	CN	No visible coating or infilling
Stain	SN	No visible coating but surfaces are discoloured by staining, often limonite (orange-brown)
Veneer	VNR	A visible coating of soil or mineral substance, usually too thin to measure (< 1 mm); may be patchy

DEFECT APERTURE

Aperture	Abbr.	Description
Closed	CL	Closed.
Open	OP	Without any infill material.
Infilled	-	Soil or rock i.e. clay, silt, talc, pyrite, quartz, etc.

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APPENDIX B – LABORATORY TEST RESULTS

STS GEOTECHNICS PTY LTD
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14/1 Cowpasture Place, Wetherill Park NSW 2164
Phone: (02)9756 2166 | Email: enquiries@stsgeo.com.au

Shrink Swell Index Report

Project: 310-314 SWAN & 984-988 CORELLA STREETS, NORTH ALBURY
Client: NSW LAND & HOUSING CORPORATION
Address: 4 Parramatta Square, 12 Darcy Street, Parramatta
Test Method: AS 1289.7.1.1

Project No.: 32427
Report No.: 23/3566
Report Date: 23/10/2023
Page: 1 OF 1

Sampling Procedure: AS 1289.1.3.1 Clause 3.1.3.2 - Thin Walled Sampler

STS / Sample No.		8053D-L/1	8053D-L/2	8053D-L/3	8053D-L/4		
Sample Location		Borehole 1 Refer to Drawing No.23/3457	Borehole 2 Refer to Drawing No.23/3457	Borehole 3 Refer to Drawing No.23/3457	Borehole 4 Refer to Drawing No.23/3457		
Material Description		Sandy Clay, orange brown/yellow (CL)	Silty Clay, red/grey brown (CL)	Silty Sandy Clay, yellow brown (CL)	Silty Sandy Clay, yellow brown (CL)		
Depth (m)		0.6 - 0.9	0.6 - 0.9	0.7 - 1.0	0.7 - 1.0		
Sample Date		9/10/2023	9/10/2023	9/10/2023	9/10/2023		
Shrink	Moisture Content (%)	18.6	19.1	21.8	19.4		
	Soil Crumbling	Nil	Nil	Nil	Nil		
	Extent of Cracking	Fine Cracks	Fine Cracks	Open Cracks	Fine Cracks		
	Strain (%)	2.9	3.7	2.2	4.1		
Swell	Moisture Content Initial (%)	18.3	15.5	22.5	17.8		
	Moisture Content Final (%)	22.3	19.9	23.5	20.3		
	Strain (%)	3.8	2.3	0.0	3.0		
Inert Inclusions (%)		<25	<20	<20	<20		
Shrink Swell Index (%)		**2.7**	**2.7**	**1.2**	**3.1**		

Remarks:

Approved Signatory.....

Technician: DH

Orlando Mendoza - Laboratory Manager

STS GEOTECHNICS PTY LTD
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STS Geotechnics Pty Ltd
14/1 Cowpasture Place, Wetherill Park NSW 2164
Phone: (02)9756 2166 | Email: enquiries@smectesting.com.au

California Bearing Ratio Determination Report

Project: 310-314 SWAN & 984-988 CORELLA STREETS, NORTH ALBURY
Client: NSW LAND & HOUSING CORPORATION
Address: 4 Parmatta Square, 12 Darcy Street, Paramatta
Test Method: AS 1289.5.1.1,6.1.1,2.1.1
No. of Days Soaked: 4

Project No.: 32427
Report No.: 23/3591
Report Date: 23/10/2023
Page: 1 of 1
Compactive Effort: Standard
Target Compaction (%): 100
Surcharge (Kg): 4.5

Sampling Procedure: AS 1289.1.2.1 Clause 6.5.3 - Power Auger Drilling (Not covered under NATA Scope of Accreditation)

STS / Sample No.		8053D-L/1					
Sample Location		Borehole 5 Refer to Drawing No.23/3457					
Material Description		Silty Clay, grey brown, with gravel (CH)					
Depth of Sample (m)		0.5 - 0.8					
Sample Date		10/10/2023					
Oversize on Wet Basis +19mm (%)		0.0					
Field Moisture Content (%)		21.2					
Optimum Moisture Content (%)		18.9					
Maximum Dry Density (t/m³)		1.74					
Dry Density (t/m³)	Before Soaking	1.75					
	After Soaking	1.73					
Relative Compaction (%)	Before Soaking	100.6					
	After Soaking	99.6					
Moisture Content (%)	Before Soaking	18.8					
	After Soaking	19.1					
Moisture Ratio Before Soaking (%)		99.5					
Moisture Content after test (%)	Top 30mm	20.7					
	Entire Depth	21.7					
Swell after Soaking (%)		1.1					
CBR Value (%)		**5.0**					
Penetration (mm)		**2.5**					

Remarks: +19mm material excluded from test

Approved Signatory.....

Technician: JC/GP

Orlando Mendoza - Laboratory Manager

CERTIFICATE OF ANALYSIS

Work Order	: **ES2334942**	Page	: 1 of 5
Client	: **STS Geotechnics**	Laboratory	: Environmental Division Sydney
Contact	: ENQUIRES STS	Contact	: Customer Services ES
Address	: Unit 14/1 Cowpasture Place Wetherill Park 2164	Address	: 277-289 Woodpark Road Smithfield NSW Australia 2164
Telephone	: ----	Telephone	: +61-2-8784 8555
Project	: 30055/31354/32388/32421/32427	Date Samples Received	: 11-Oct-2023 12:45
Order number	: 2023-353	Date Analysis Commenced	: 13-Oct-2023
C-O-C number	: ----	Issue Date	: 17-Oct-2023 18:01
Sampler	: IS, MB, PS		
Site	: ----		
Quote number	: EN/222		
No. of samples received	: 15		
No. of samples analysed	: 15		

Accreditation No. 825
Accredited for compliance with
ISO/IEC 17025 - Testing

This report supersedes any previous report(s) with this reference. Results apply to the sample(s) as submitted, unless the sampling was conducted by ALS. This document shall not be reproduced, except in full.

This Certificate of Analysis contains the following information:

- General Comments
- Analytical Results

Additional information pertinent to this report will be found in the following separate attachments: Quality Control Report, QA/QC Compliance Assessment to assist with Quality Review and Sample Receipt Notification.

Signatories

This document has been electronically signed by the authorized signatories below. Electronic signing is carried out in compliance with procedures specified in 21 CFR Part 11.

Signatories	*Position*	*Accreditation Category*
Ankit Joshi	Senior Chemist - Inorganics	Sydney Inorganics, Smithfield, NSW
Franco Lentini	LCMS Coordinator	Sydney Inorganics, Smithfield, NSW

General Comments

The analytical procedures used by ALS have been developed from established internationally recognised procedures such as those published by the USEPA, APHA, AS and NEPM. In house developed procedures are fully validated and are often at the client request.

Where moisture determination has been performed, results are reported on a dry weight basis.

Where a reported less than (<) result is higher than the LOR, this may be due to primary sample extract/digestate dilution and/or insufficient sample for analysis.

Where the LOR of a reported result differs from standard LOR, this may be due to high moisture content, insufficient sample (reduced weight employed) or matrix interference.

When sampling time information is not provided by the client, sampling dates are shown without a time component. In these instances, the time component has been assumed by the laboratory for processing purposes.

Where a result is required to meet compliance limits the associated uncertainty must be considered. Refer to the ALS Contract for details.

Key : CAS Number = CAS registry number from database maintained by Chemical Abstracts Services. The Chemical Abstracts Service is a division of the American Chemical Society.
LOR = Limit of reporting
^ = This result is computed from individual analyte detections at or above the level of reporting
ø = ALS is not NATA accredited for these tests.
~ = Indicates an estimated value.

- ED045G: The presence of Thiocyanate, Thiosulfate and Sulfite can positively contribute to the chloride result, thereby may bias results higher than expected. Results should be scrutinised accordingly.

Analytical Results

Sub-Matrix: SOIL (Matrix: SOIL)				Sample ID	30055/9196	31354/145	31354/146	32388/BH2-S1	32388/BH3-S2
				Sampling date / time	10-Oct-2023 00:00	10-Oct-2023 00:00	10-Oct-2023 00:00	09-Oct-2023 00:00	09-Oct-2023 00:00
Compound	CAS Number	LOR	Unit		ES2334942-001	ES2334942-002	ES2334942-003	ES2334942-004	ES2334942-005
					Result	Result	Result	Result	Result
EA002: pH 1:5 (Soils)									
pH Value	----	0.1	pH Unit		5.7	6.1	5.8	5.9	6.4
EA010: Conductivity (1:5)									
Electrical Conductivity @ 25°C	----	1	µS/cm		49	66	169	76	154
EA055: Moisture Content (Dried @ 105-110°C)									
Moisture Content	----	0.1	%		13.5	15.1	13.6	11.4	13.7
ED040S : Soluble Sulfate by ICPAES									
Sulfate as SO4 2-	14808-79-8	10	mg/kg		50	50	90	70	110
ED045G: Chloride by Discrete Analyser									
Chloride	16887-00-6	10	mg/kg		----	----	----	100	170

Analytical Results

Sub-Matrix: SOIL (Matrix: SOIL)			Sample ID	32388/BH4-S3	32388/BH6-S4	32388/BH7-S5	32421/S1	32421/S2
			Sampling date / time	09-Oct-2023 00:00	09-Oct-2023 00:00	09-Oct-2023 00:00	09-Oct-2023 00:00	09-Oct-2023 00:00
Compound	CAS Number	LOR	Unit	ES2334942-006	ES2334942-007	ES2334942-008	ES2334942-009	ES2334942-010
				Result	Result	Result	Result	Result
EA002: pH 1:5 (Soils)								
pH Value	----	0.1	pH Unit	5.2	6.2	6.4	7.0	5.9
EA010: Conductivity (1:5)								
Electrical Conductivity @ 25°C	----	1	µS/cm	287	36	36	45	55
EA055: Moisture Content (Dried @ 105-110°C)								
Moisture Content	----	0.1	%	17.4	15.2	15.4	6.5	11.8
ED040S : Soluble Sulfate by ICPAES								
Sulfate as SO4 2-	14808-79-8	10	mg/kg	310	40	40	10	50
ED045G: Chloride by Discrete Analyser								
Chloride	16887-00-6	10	mg/kg	300	60	90	10	20

Analytical Results

Sub-Matrix: SOIL (Matrix: SOIL)			Sample ID	32421/S3	32427/S1	32427/S2	32427/S3	32427/S4
			Sampling date / time	09-Oct-2023 00:00	09-Oct-2023 00:00	09-Oct-2023 00:00	09-Oct-2023 00:00	09-Oct-2023 00:00
Compound	CAS Number	LOR	Unit	ES2334942-011	ES2334942-012	ES2334942-013	ES2334942-014	ES2334942-015
				Result	Result	Result	Result	Result
EA002: pH 1:5 (Soils)								
pH Value	----	0.1	pH Unit	**7.2**	**6.3**	**6.1**	**6.5**	**6.6**
EA010: Conductivity (1:5)								
Electrical Conductivity @ 25°C	----	1	µS/cm	**263**	**22**	**24**	**11**	**23**
EA055: Moisture Content (Dried @ 105-110°C)								
Moisture Content	----	0.1	%	**9.7**	**9.9**	**11.7**	**13.2**	**17.4**
ED040S : Soluble Sulfate by ICPAES								
Sulfate as SO4 2-	14808-79-8	10	mg/kg	**60**	<10	<10	<10	<10
ED045G: Chloride by Discrete Analyser								
Chloride	16887-00-6	10	mg/kg	**140**	<10	<10	<10	<10